LOW CARBON NOW
for Multi-Unit Residential
Developments in Toronto

Building Smarter Structures: A Study in
Low Embodied Carbon Structural Strategies

## Embodied Carbon <br> of Baseline Building

The structure of a typical residential tower can account for over 60\% of the total embodied carbon of the building. Therefore, optimizing the structural layout to reduce quantities and minimize embodied emission is fundamental for our path towards Net Zero. The following study was developed to evaluate the embodied carbon of various structural systems and determine the optimal approach to a low-carbon pathway. A baseline building was developed, representing typical construction and design practices for a concrete residential tower in Toronto, in order to serve as a baseline for comparison.

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Building Archetype
Building Archetype
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40-storey multi-unit residential building in Toronto
6 levels of podium
4 levels of below-grade parking

- 1800 mm transfer slab at level 2
Shear walls and columns vertical structure
. 200 mm flat slabs
Conventional concrete mix design
Cantilever balconies


## Foundation Design Assumptions

The below-grade structure was designed as a tanked foundation system to meet the City of Toronto Foundation Drainage Provisions. The following assumptions were made on the soil properties
(4) Groundwater table is between P1 and P2 elevation.

Bearing capacity of soil/rock: 1100 kPa ULS.
No surcharge loading from adjacent buildings.
Embodied Carbon Impact of Balconies
The baseline structure was also designed without balconies to understand the embodied carbon emissions of the balconies.
Results: Total GWP Intensity (A-C) of the building structure without balconies: 409 $\mathrm{kgCO} \mathrm{C}^{2} / \mathrm{m}^{2}$
Conclusion: Balconies add roughly $3 \%$ embodied carbon to the structure.
Note: the various structural studies presented in this document did not include balconies.

Embodied Carbon Results

| Life-Cycle Stage | GWP | GWP Intensity |
| :---: | :---: | :---: |
|  | (tonnes $\mathrm{CO}_{2} \mathrm{e}$ ) | ( $\mathrm{kg} \mathrm{CO}_{2} \mathrm{e} / \mathrm{m}^{2}$ ) |
| A1-A3-Construction Materials | 9,890 | 352 |
| A4 - Transportation to Site | 203 | 7 |
| A5 - Construction/Installation Process | 1,293 | 46 |
| C1- Deconstruction/Demolition | 95 | 3 |
| C2 - Waste Transportation | 221 | 8 |
| C3 - Waste Processing | 7 | 0 |
| C4 - Waste Disposal | 114 | 4 |
| Total A-C | 11,823 | 421 |



Embodied Carbon Distribution per Structural Element





## Slab Strategies

EN for the Tower

Slabs typically account for about 40\% of the total embodied carbon in the structure, serving as a carbon sink in the building. As such, reducing material quantities in the slabs will have the greatest impact on reducing emissions, but will also reduce the loads carried from the slabs down to the carried from the slabs down to the
foundations. In this study we evaluated two slab construction strategies for the two slab construction strategies for the 200 mm reinforced concrete slab, which reduced the slab thickness

## Embodied Carbon Analysis

Tower layout and structural propertiesVertical Elements = Discrete Columns

- Grid $=6.5 \mathrm{~m} \times 6.5 \mathrm{~m}$

Floor plate $=25 \mathrm{~m} \times 30 \mathrm{~m}$
No. of Tower Stories $=33$

- Slab $f^{\prime} \mathrm{c}=35 \mathrm{MPa}$

Column \& Core Wall $f^{\prime} \mathrm{c}=35-60 \mathrm{MPa}$
LCA Scope: Modules A1-A5, C1-C4

Construction Strategies for Slab Thickness - Tower

|  | Slab System | Slab <br> Thickness <br> $(\mathrm{mm})$ |  | Comments |
| :--- | :---: | :---: | :---: | :---: |
| Baseline | Reinforced Concrete <br> Slab | 200 | Typical slab system in Toronto. |  |
| AchievableReinforced Concrete <br> Slab- Backshoring <br> Method | 180 | Reduction in slab thickness was achieved on the same grid layout with a reshoring <br> procedure practiced in Western Canada where the cast slab is cured to $70 \%$ of the <br> specified design strength or 25MPa minimum, upon which the formwork is sequentially <br> removed and backshored inmediately. |  |  |
| Design <br> Limit | Post-Tensioned Slab | 165 | Slab thickness is fairly thin and can be impractical. Post-tensioned slabs are typically <br> used to get longer spans for the same slab depth. A 165 mm PT slab is doable but acoustic <br> issues arise which need to be addressed. |  |

Embodied Carbon Results - Tower as a Whole

| Slab System | GWP | GWP Intensity |
| :--- | :---: | :---: |
|  | (tonnes $\left.\mathrm{CO}_{2} \mathrm{e}\right)$ | $\left(\mathrm{kg} \mathrm{CO}_{2} \mathrm{e} / \mathrm{m}^{2}\right)$ |
| Baseline -200 RC Slab | 4,968 | 221 |
| Achievable -180 RC Backshoring* | 4,740 | 210 <br> $(-5 \%)$ |
| Design Limit - 165 PT Slab** | 4,479 | 199 <br> $(-10 \%)$ |

Fig. 2 Scope of Slab \& Column Layout Strategy for EC Analysis

Embodied Carbon Results - Tower Slabs Only

| Slab System | GWP | GWP Intensity |
| :--- | :---: | :---: |
|  | $\left(\mathrm{kg} \mathrm{CO}_{2} \mathrm{e}\right)$ | $\left(\mathrm{kg} \mathrm{CO}_{2} \mathrm{e} / \mathrm{m}^{2}\right)$ |
| Baseline -200 RC Slab | 2,683 | 119 |
| Achievable - 180 RC Backshoring* | 2,457 | 109 <br> $(-8 \%)$ |
| Design Limit - 165 PT Slab** $^{*}$ | 2,201 | 98 <br> $(-18 \%)$ |

*Acoustic elements for a 180 mm slab would add about $115,914 \mathrm{~kg} \mathrm{CO}_{2}$ e or $5.1 \mathrm{~kg} \mathrm{CO}, \mathrm{e} / \mathrm{m}^{2}$. *Acoustic elements for a 165 mm slab would add about $215,054 \mathrm{~kg} \mathrm{CO}_{2}$ e or $9.5 \mathrm{~kg} \mathrm{CO}_{2} 2 / \mathrm{m}^{2}$.

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Slab Study Takeaways
Reducing the slab thickness below 200 mm introduces acoustic requirements. The added embodied carbon from the acoustic elements cuts the percent reduction of the thinner slabs by half.

Construction of typical reinforced concrete slabs in Western Canada typically practice reshoring method. Trades in the GTHA do not practice this method and we therefore need trades onboard very early in the project to achieve this.
Post-tensioned slabs are not standard construction in Ontario. Could be difficult to obtain trades and costly to execute

Thin slabs (i.e, 165 mm ) can be impractical, such as introduce limitations on slab anchorage.

Fig. 1 Tower Grid Layout


## Column Layout <br> Strategies for Tower

The column layout is mainly defined based on the architectural program Typical residential buildings have a grid layout of 6.5 m by 6.5 m for $a^{2} 200 \mathrm{~mm}$ slab. The following study was conducted to evaluate the embodied carbon impact of two larger grid layouts in order to determine the optimal approach for the suite layout.

Embodied Carbon Analysis
Tower layout and structural properties:
(O) Vertical Elements $=$ Discrete Columns
Slab thickness $=200 \mathrm{~mm}$
Slabf'c $=35 \mathrm{MPa}$

- Floor plate $=25 \mathrm{~m} \times 30 \mathrm{~m}$
- Column \& Core Wall f'c $=35-60 \mathrm{MPa}$
LCA Scope: Modules A1-A5, C1-C4



Fig. 4 Achievable Tower Grid Layout
7.5 m by 7.5 m


Fig. 5 Design Limit Tower Grid Layout -

No. of Tower Stories $=33$

Design Strategies for Column Layout - Tower
Embodied Carbon Results - Tower as a Whole

| Slab System |  | Grid Layout | Comments |
| :---: | :---: | :---: | :---: |
| Baseline | Reinforced Concrete Slab | $6.5 \mathrm{~m} \times 6.5 \mathrm{~m}$ | Typical slab and grid system in Toronto. |
| Achievable | Reinforced Concrete Slab-Backshoring Method | 7.5mx7.5m | Reduction in slab thickness was achieved on the same grid layout with a reshoring procedure practiced in Western Canada where the cast slab is cured to $70 \%$ of the specified design strength or 25 MPa minimum, upon which the formwork is sequentially removed and backshored immediately. |
| Design Limit | Post-Tensioned Slab | $8.5 \mathrm{~m} \times 8.5 \mathrm{~m}$ | Post-tensioned slab construction is atypical for Toronto residential buildings. |

## Column Layout Study Takeaway

Achievable grid layout is limited to the architectural program, i.e., suite layouts, parking layouts, etc.

There is minimal embodied carbon impact to adjusting the grid layout for this specific study.
The Achievable slab system demonstrates a slight increase in embodied carbon due to the higher steel reinforcement density required in the slabs to accommodate the longer spans. The additional rebar adds more GWP than the savings achieved in the columns.

## Lateral System <br> Strategies for Tower

Structural walls are estimated to account for up to $25 \%$ of the total embodied carbon in the structure. Therefore, designing a lateral system that minimizes walls can have a considerable impact on the embodied emissions. Typical construction practice in a residential building in Toronto consists of walls between suite and a central core. In this study we evaluated the embodied carbon impact of two alternative approaches, which consisted of wallumns with a central core and fin wall, and columns with a central core and fin wall.

Lateral System Options - Tower

Option 1: Shear Walls between Suites


Option 2: Wallumns


Embodied Carbon Results - Tower as a Whole

| Lateral System | GWP Structure | GWP Intensity Structure | GWP Intensity Demising Walls | GWP Intensity Demising Walls \& Structure |
| :---: | :---: | :---: | :---: | :---: |
|  | (tonnes $\mathrm{CO}_{2} \mathrm{e}$ ) | (kg CO2 $\mathrm{e}^{\left(\mathrm{m}^{2}\right)}$ | ( $\mathrm{kg} \mathrm{CO}_{2} \mathrm{e} / \mathrm{m}^{2}$ ) | ( $\mathrm{kg} \mathrm{CO}_{2} \mathrm{e} / \mathrm{m}^{2}$ ) |
| 1. Shear Walls between Suites | 5,439 | 241 |  | 241 |
| 2. Wallumns | 5,281 | $\begin{gathered} 234 \\ (-2.9 \%) \end{gathered}$ | +2.0 | $\begin{gathered} 237 \\ (-2.1 \%) \end{gathered}$ |
| 3. Discrete Columns | 5,014 | $\begin{gathered} 223 \\ (-7.8 \%) \end{gathered}$ | +3.9 | $\begin{gathered} 227 \\ (-6.2 \%) \end{gathered}$ |

Embodied Carbon of Tower Structure per Lateral System




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## Lateral System Study Takeaways

Architectural demising walls are required between suites in lieu of shear walls, The addition of demising walls introduces a slight increase in embodied carbon to the building.

## Transfer Options

## for the Podium

Transfer systems are both costly and carbon-heavy due to the large volume of reinforced concrete required to transition the loads between floors. The optim solution is to avoid or limit transfers altogether, ideally by having the vertical elements extend from top to bottom without interruption. However, residentia buildings in Toronto typically have at least one transfer floor which is often in the form of a transfer slab

Orthogonal Transfer


оritoconal L Lab bano transfer system

Non-Orthogonal Transfer Layout


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non-orthogonal transfer s sLa system

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NON-ORTHOGONAL WALL BEAM TRANSEER SSSTEM

Structural Properties

| Transfer System Parameters |  |  |  |
| :---: | :---: | :---: | :---: |
| Bay Size 9 | $9 \mathrm{~m} \times 9 \mathrm{~m}$ |  |  |
| Floor Plate 11.4 | $11.4 \mathrm{~m} \times 11.4 \mathrm{~m}$ |  |  |
| Typical Slab Thickness 30 | 300 mm |  |  |
| Transfer Slab Thickness 1 | $1200 \mathrm{~mm}^{*}$ |  |  |
| Slab \& Transfers f'c 3 | 35 MPa |  |  |
| Transfer Load 1200 | $12,000 \mathrm{kN}$ (typical for 25 to 30-storey residential tower) |  |  |
| *Original study was conducted for a 20-storey residential tower. The transfer slab for a 40-storey tower would be about 1800 mm deep. |  |  |  |
| Embodied Carbon Results |  |  |  |
| Transfer Layout | A1-A3 GWP | Total A-C* GWP |  |
|  | $\left(\mathrm{kg} \mathrm{CO} 2 \mathrm{e} / \mathrm{m}^{2}\right)$ | $\left(\mathrm{kg} \mathrm{CO} 2 \mathrm{e} / \mathrm{m}^{2}\right)$ | \% Difference |
| Orthogonal Slab Band | 494 | 583 | +62\% |
| Orthogonal Transfer Slab | b 706 | 811 | +126\% |
| Orthogonal Wall Beam | 295 | 359 | Lowest EC |
| Non-Orthogonal Slab Band | and 734 | 844 | +135\% |
| Non-Orthogonal Transfer Slab | - 719 | 826 | +130\% |
| Non-Orthogonal Wall Beam | eam 444 | 524 | +46\% |

*Includes the following life-cycle stages: A1-A3, A4, A5, C1-C4.

Transfer Study Takeaways
The viable transfer system is highly dependent on the grid layout.
Wall beams prevent access to areas, introducing architectural limitations. Additionally, non-orthogonal wall beam systems are impractical as they create inaccessible areas.

Greatest impact on reducing embodied carbon with transfer systems is achieved by limiting transfers to a single floor \& minimizing the number of transfers.

## Below-Grade

## Parking Options

Below-grade parking structures add a significant amount of embodied carbon to the building due to the volume of concrete required to construct them. Typical residential towers in Toronto have four levels of parking, which was estimated to account for a quarter (28\%) of the total embodied carbon of the baseline building structure in this research program. Therefore, the following study evaluates the embodied carbon impact of reducing the number of parking levels and the contribution these alternative options have on the building as a whole.

Embodied Carbon Results - Whole Building

| Underground Parking System | Total A-C GWP |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: |
|  | GWP <br> (tonnes $\left.\mathrm{CO}_{2} \mathrm{e}\right)$ | GWP Intensity <br> $\left(\mathrm{kg} \mathrm{CO}_{2} \mathrm{e} / \mathrm{m}^{2}\right)$ | \% Difference <br> from Option 1 | \% of Below-Grade <br> Structure <br> Contriution |  |
| Option 1 | 4 Levels Below-Grade | 11,477 | 409 | - | $28 \%$ |
| Option 2 | 3 Levels Below-Grade | 11,010 | 392 | $-4 \%$ | $25 \%$ |
| Option 3 | 2 Levels Below-Grade | 10,611 | 378 | $-8 \%$ | $22 \%$ |
| Option 4 | 1 Level Below-Grade | 9,288 | 331 | $-12 \%$ | $11 \%$ |
| Option 5 | No Below-Grade Parking | 9,246 | 329 | $-13 \%$ | $10 \%$ |

Foundation Design Assumptions
The below-grade structure was designed as a tanked foundation system to meet the City of Toronto Foundation Drainage Provisions. The following assumptions were made on the soil properties:
(C) Groundwater table is between P1 and P2 elevation.

For no parking, structural tanking is not required as it is above the groundwater table.
Bearing capacity of soil/rock: 1100 kPa ULS.
Soil/rock bearing is valid for foundation depths associated with 1 level of underground parking.

Assumed that bottom of footings for the no below grade parking option is about 4.4 m below ground floor.
No surcharge loading from adjacent buildings.


Embodied Carbon of Whole Structure per Below-Grade Structure


## Low-Carbon Pathways

Low-Carbon Pathways
The results of the various structural studies were used to determine low-carbon pathways for the full building structure. Two pathways were selected:

| Pathway | Structural Systems | Comments |
| :---: | :---: | :---: |
| Ideal | 165 mm PT slab with 4 " insulation and 2 layers of GWB for acoustic requirements No balconies Discrete columns with architectural demising walls at a $6.5 \mathrm{~m} \times 6.5 \mathrm{~m}$ grid, a central core and fin wall. Transfers at Level 2 only made up of transfer beams and a transfer slab for the scissor stair. One below-grade level. | This option is not considered realistic due to the acoustic requirements of the thin slab. One below-grade parking level was selected since no below-grade structure was deemed unrealistic for a building of this typology. |
| Practical | 200 mm RC slab <br> No balconies Discrete columns with architectural demising walls at a $6.5 \mathrm{~m} \times 6.5 \mathrm{~m}$ grid, a central core and fin wall. Transfers at Level 2 only made up of transfer beams and a transfer slab for the scissor stair. One below-grade level. | Although a PT slab system would achieve a larger grid layout, resulting in $2 \%$ embodied carbon savings, it is not standard construction in Ontario. Therefore, due to the minimal carbon reductions and added construction complexity, a standard reinforced concrete slab was selected. |

Embodied Carbon of Whole Structure per Pathway


Embodied Carbon Results

| Building System | GWP | GWP Intensity |
| :--- | :---: | :---: |
|  | (tonnes $\left.\mathrm{CO}_{2} \mathrm{e}\right)$ | $\left(\mathrm{kg} \mathrm{CO}_{2} \mathrm{e} / \mathrm{m}^{2}\right)$ |
| Baseline (with balconies) | 11,823 | 421 |
| Ideal Pathway | 7,659 | 273 <br> $(-35 \%)$ |
| Practical Pathway | 8,545 | 304 <br> $(-28 \%)$ |

> Low Carbon Design Pathway Takeaways

Good, Better, \& Best embodied carbon targets were set to align with IStructE's SCORS C, B, \& A ratings for LCA modules A1-A5.

- Good "C" rating $=200-250 \mathrm{~kg}$ CO2e/m2.

Better " B " rating $=150-200 \mathrm{~kg}$ CO2e/m2.
Best "A" rating $=100-150 \mathrm{~kg}$ CO2e $/ \mathrm{m} 2$.

The estimated GWP of all three designs for A1-A5 emissions is as follows:
$406 \mathrm{kgCO} 2 \mathrm{e} / \mathrm{m} 2$ for the Baseline,
$262 \mathrm{kgCO} 2 \mathrm{e} / \mathrm{m} 2$ for the Ideal Pathway
$293 \mathrm{kgCO} 2 \mathrm{e} / \mathrm{m} 2$ for the Realistic Pathway.
All of the designs exceeded the "C" rating (>250 kgCO2e/m2).

## Low-Carbon

## Material Pathways

Low-Carbon Material Pathways
The embodied carbon of the structure can be further reduced by selecting low-carbon materials. Several low-carbon iterations (LCI) were applied to the Ideal Pathway with the results summarized below. The description for the LCl's defines the changes from the baseline, all other conditions are the same.

| Case | Description | GWP Intensity | Difference from Baseline |
| :---: | :---: | :---: | :---: |
|  |  | ( $\mathrm{kg} \mathrm{CO}_{2} \mathrm{e} / \mathrm{m}^{2}$ ) |  |
| Ideal Pathway (Baseline) | Industry average concrete mixes consisting of General Use Portland Cement (GU) and 10\% supplementary cementitious materials (SCM) content. Industry average reinforcing bars. | 273 | - |
| LCI-01 | Concrete mixes with GU cement and 35-50\% slag cement. | 235 | -14\% |
| LCI-02 | Concrete mixes with General Use Portland Limestone (GUL) cement and 35-50\% slag cement. | 222 | -18\% |
| LCI-03 | Lafarge's ECOPact Entry Level low-carbon concrete. | 215 | -21\% |
| LCI-04 | Lafarge's ECOPact Prime low-carbon concrete. | 186 | -32\% |
| LCI-05 | Lower-carbon rebar: Gerdau's rebar produced at the Whitby, ON plant. | 266 | -2\% |
| Max LCI | LCI-04 and LCI-05 | 181 | -34\% |

Note:

- The LCIs are limited based on available lifecycle inventory data. For instance, another LCI would be a standard concrete mix with GUL cement. However, the EPD used for the analysis did not include this datapoint so it was not analyzed. Studies show that concrete with GUL cement has about $10 \%$ less embodied carbon than with GU cement.

All GWP values in the analysis represent current conditions and should be updated as more relevant data becomes available.

Low-Carbon Material Takeaways
High SCM \& low-carbon concrete products have performance implications, which could affect the construction schedule

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| :---: | :---: | :---: | :---: |
|  |  | ( $\mathrm{kg} \mathrm{CO}_{2} \mathrm{e} / \mathrm{m}^{2}$ ) |  |
| Practical Pathway (Baseline) | Industry average concrete mixes consisting of General Use Portland Cement (GU) and 10\% supplementary cementitious materials (SCM) content. Industry average reinforcing bars. | 304 | - |
| LCI-01 | Concrete mixes with GU cement and 35-50\% slag cement. | 262 | -14\% |
| LCI-02 | Concrete mixes with General Use Portland Limestone (GUL) cement and 35-50\% slag cement. | 249 | -18\% |
| LCI-03 | Lafarge's ECOPact Entry Level low-carbon concrete. | 240 | -21\% |
| LCI-04 | Lafarge's ECOPact Prime low-carbon concrete. | 209 | -31\% |
| LCI-05 | Lower-carbon rebar: Gerdau's rebar produced at the Whitby, ON plant. | 296 | -3\% |
| Max LCI | LCI-04 and LCI-05 | 200 | -34\% |

Note:

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